ROCKY MOUNTAIN
INDUSTRIALS

RMR Aggregates, Inc.
6200 S. Syracuse Way, Suite 450
Greenwood Village, CO 80111

October 3, 2025

Colorado Division of Reclamation, Mining and Safety
Room 215, c/o: Amy Yeldell
1001 E. 62nd Avenue
Denver, CO 80216

RE: Mid-Continent Quarry Technical Revision (TR-9)

Dear Ms. Yeldell,

RMR Aggregates, Inc. (RMRA) is submitting the following version of Technical Revision (TR-9) in compliance with the CODRMS's request to "provide a solution to highwall stabilization within the current DRMS boundary that could be implemented immediately in the event of revocation and forfeiture." The included highwall stabilization reclamation plan serves as an interim addition to the reclamation plan prior to a future amendment of the broader mining permit. The technical revision, which is comprised of additions to the reclamation plan, and supporting information pertaining to the plan are contained in the following pages.

The contents of the following pages are summarized as follows:

1. Technical Revision Reclamation Plan
2. Rationale for the Technical Revision methodology
3. "Highwall Reclamation Stabilization" memorandum by Golden Geotechnics Inc.
4. Request to accept and justification for accepting 1.15 pseudostatic Factor of Safety, "Highwall Reclamation Seismic Factor of Safety" memorandum by Golden Geotechnics Inc.
5. Geotechnical model calculations
6. Rock bolting quotes

Please feel free to reach out with any questions regarding this technical revision.

Sincerely,

Robert T. Wagner
VP, Engineering
Rocky Mountain Industrials
rwagner@rockymountainindustrials.com

Reclamation Plan in Case of Revocation and Forfeiture

In the case of revocation and forfeiture of the Mid-Continent Quarry, the following highwall stabilization measures will be added to the quarry reclamation plan. In any case other than revocation and forfeiture, the standard reclamation plan will be followed. Additionally, the use of highwall stabilization in reclamation may change or become unnecessary as mining at the quarry progresses, new geological data is obtained pertaining to the highwall stability, or conditions change on site.

Highwall Geology

The Mid-Continent Quarry highwall is approximately 900 feet in length. In January 2023, approximately 400 feet of the western section of the highwall experienced a rockfall event where some of the material failed along a failure plane in the rock formation. The failure propagated up the slope until it reached the natural fracture planes of the limestone formation. Since the time of the failure, no additional failures have occurred at the quarry (as of October 2025). The eastern half of the highwall has not experienced a failure of any kind as of the writing of this reclamation plan addition (October 2025). The plane found in the highwall associated with the failure has a dip angle of [redacted] on average, with the eastern portion of the highwall having the lower dip angle, and the western portion of the highwall having the higher dip angle. Additionally, the dip angle decreases as you progress uphill in the formation.

Geotechnical Analysis

Utilizing field measurements, backanalysis, and computer-based modeling and slope stability analysis, RMRA's geotechnical engineers were able to determine the locations of the quarry highwall in need of stabilization as a part of reclamation. They were also able to determine the necessary stabilization measures to ensure long-term stabilization of the specific highwall areas. See the referenced "Highwall Reclamation Stabilization" letter provided by Golden Geotechnics, Inc.

Areas Requiring Highwall Reclamation Stabilization

Through the geotechnical analysis process, it was determined that the western section of the quarry highwall required stabilization as a part of reclamation. It was also determined that the eastern section of the highwall did not require stabilization. The total length of the western highwall requiring stabilization is approximately 450 feet. The highwall stabilization area is shown in the figure below.

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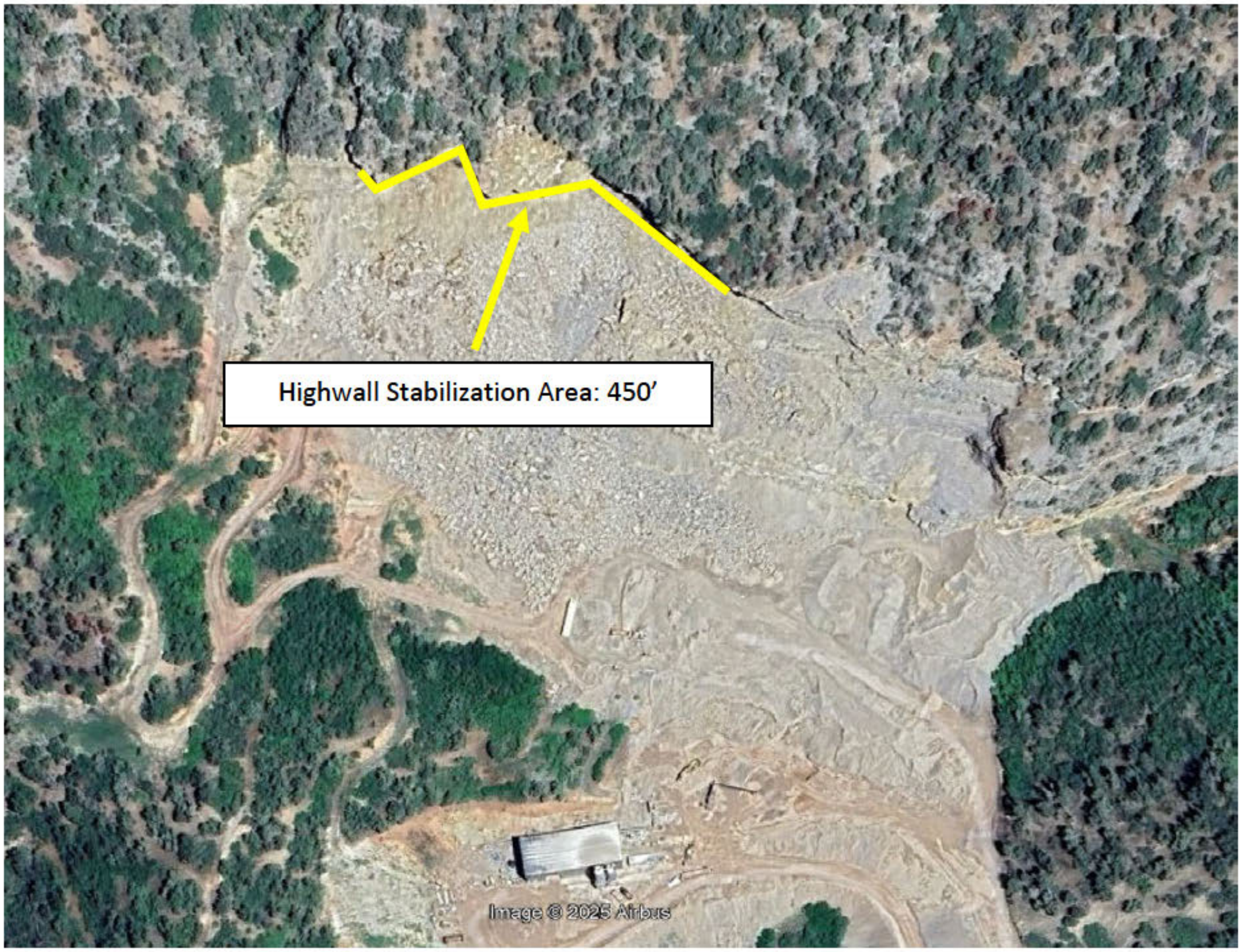


Highwall Stabilization Area

Highwall Stabilization Method

The highwall stabilization area shown above will be stabilized using mechanical stabilization in the form of rock bolting. Rock bolting will be completed utilizing [redacted] Bolt installation specifics are as follows:

[redacted]	[redacted]	[redacted]	[redacted]	[redacted]	[redacted]
[redacted]	[redacted]	[redacted]	[redacted]	[redacted]	[redacted]

Alternative rock bolting methods may be utilized so long as they meet or exceed the stabilization performance of the above-mentioned anchors.

Technical Revision 9 (TR-9) Rationale

Parameters and Calculations

Calculation parameters were derived from back analysis using the known factors that contributed to the rockfall event. Dip angles, friction angles, and cohesion values were either observed in the field or calculated based on empirical parameters. For modeling the stabilization of the West face, multiple levels of conservatism were introduced to exceed the minimum FOS specified by CODRMS. Several back analyses were performed using the geometry of the highwall at the 2023 failure event to verify the selected rock mass input parameters. The initial back analysis used a steeper slope to mimic the limestone bed rolling on the face of the monocline to a steeper angle, which would be more in line with the highwall face at the time and location of the event. A second analysis was performed using a reduced slope of 30 degrees that would mimic the slope at the top of the failure, well above the point of release. Slope stability analyses were performed using the software program RocPlane from RocScience (V.4.0111). The back analysis was verified by running the analysis again in a separate slope stability program called Slope/W.

Reasons for the Non-Critical Structures Designation

During our analysis for this TR-9, the reclaimed quarry and rock bolted highwall was viewed as having a “Non-Critical Structures” designation, per the definitions found in the Colorado Department of Reclamation, Mining and Safety, 2018, Policies of the Mined Land Reclamation Board, dated May 16, 2018, 14pp. By the definition in that document, “Non-Critical Structures” include limited structures, such as fences, and present "no imminent danger to human life”. The stabilized slope will not be near structures of any kind. By contrast, the “Critical Structures” designation correlates with residences, utilities, dams, public roads, etc. These structures would create a likelihood of human proximity to the slope during a failure.

The reclaimed quarry poses no imminent danger to human life because of the following.

- All access roads to the quarry area will be blocked with large rocks, as a part of the reclamation process, to prevent driving on site.
- The area directly below the section of the highwall to be rock bolted will not contain any roads or trails. It will not be flat or conducive to hiking, camping, biking, or other activities.
- There are not any current points of interest for the public where the expected traffic route, on foot or otherwise, would be through the quarry. All points of interest in the area are up along Transfer Trail.
- The quarry and the section of the highwall to be rock bolted are not easily accessible from the 4x4 road above the quarry (1,000’ due north of the highwall). The area between the road and the quarry is steep and covered with brush and scree.
- Even if a slope failure were to occur, the maximum extent of the potential failure would not extend up the slope to the 4x4 road above the quarry due to the failure layer pinching out.

[redacted]

Factors of Safety

The CODRMS factors of safety for "Critical Structures" are 1.5 for static conditions and 1.3 for pseudostatic conditions. The CODRMS factors of safety for "Non-critical Structures" are 1.3 for static conditions and 1.15 for pseudostatic conditions. As described in the previous section, our analysis and stabilization plan utilize the "Non-Critical Structures" designation. When evaluating the factors of safety, it is important to note that for a reclaimed quarry, a pseudostatic condition would only occur in the case of an earthquake at the site since blasting is no longer being performed. This makes pseudostatic conditions highly unlikely, especially in the Glenwood Springs region, which is not known for its seismic activity.

Even though the "Non-Critical Structures" designation was used in our analysis, the calculated factors of safety in our TR-9 meet the "Critical Structures" levels for static and pseudostatic conditions for the East Highwall of the quarry. For the West Highwall of the quarry, and its associated rock bolting plan, our factors of safety meet the "Critical Structures" level for static conditions and the "Non-Critical Structures" level for pseudostatic conditions. Three of the four calculated factor of safety values meet the "Critical Structures" levels. Only the pseudostatic condition (earthquake) factor of safety falls short of the "Critical Structures" level while still meeting the "Non-Critical Structures" level.

We are meeting/exceeding "Critical Structures" factor of safety levels in three of the four values even though the site does not meet the definition of "Critical Structure" in its reclaimed state. We believe this shows that our analysis and suggested stabilization plan is not simply a minimum effort to correct the situation, but a genuine plan for long-term remediation. We are requesting the CODRMS accept the 1.15 Factor of Safety for pseudostatic conditions. Further support for this request is contained in the "Highwall Reclamation Seismic Factor of Safety" memo included below.

Stabilization Condition	CODRMS "Critical Structures"	CODRMS "Non-Critical Structures"	Quarry East Highwall	Quarry West Highwall
Static Factor of Safety	1.5	1.3	1.75	1.51
Pseudostatic Factor of Safety	1.3	1.15	1.38	1.15

GOLDEN GEOTECHNICS INC

April 10, 2025

Mr. Robert Wagner
RMR Aggregates, Inc.
6200 S. Syracuse Way, Ste. 450
Greenwood Village, CO 80111

RE: **Highwall Reclamation Stabilization**
Mid-Continent Limestone Quarry
Glenwood Springs, CO
GGI Project No.: 23003-04

Dear Mr. Wagner,

Golden Geotechnics, Inc. (GGI), in conjunction with Brierley Associates, Corp (BAC) is pleased to submit this memorandum regarding slope stabilization for the Mid-Continent Limestone Quarry highwall reclamation stabilization plan. Specifically, the discussion herein addresses a request by the State of Colorado Division of Reclamation, Mining and Safety (DRMS) to "provide a solution to highwall stabilization within the current DRMS boundary that could be implemented immediately in the event of revocation and forfeiture."

On behalf of RMRA, GGI and BAC reviewed the Kilduff Underground Engineering (KUE) Rock Failure Analyses and Stabilization Report dated March 5, 2024 for the applicability of using the stabilization analyses and preliminary design of active stabilization during mining operations presented in the report as the de facto reclamation procedure for the entirety of the slope as has been done by DRMS.

As part of the failure analyses, the KUE report evaluated the failure path of the slope that could impact quarry facilities on the bench and potentially impact life safety as part of active mining operations. As a result, the report considered the structures as "Critical" to life safety per DRMS MLRB policies1 and performed static and pseudostatic analyses with a minimum factor of safety of 1.5 and 1.3, respectively. The slope stability analyses were performed using some historic data provided by RMRA, but the model input parameters were largely informed by empirical values that were confirmed through a backanalysis.

The DRMS MLRB policy maintains the same minimum factors of safety for both operations and reclamation, however the Mid-Continent quarry site will no longer be considered a "Critical Structure" once operations have ceased and the potential for human safety risk has been eliminated. Therefore, the appropriate minimum factor of safety for reclamation slope stability analyses is 1.3 for static and 1.15 for pseudostatic.

A kinematic analysis was performed on the east face and west face to determine the stability of the slope and support, if any, would be required for remediation. General limit equilibrium method slope stability analyses were performed using the software program RocPlane from RocScience (V.4.0111). A factor of safety is calculated by modeling the effects of joint shear

[1] Colorado Department of Reclamation, Mining and Safety, 2018, Policies of the Mined Land Reclamation Board, dated May 16, 2018, 14pp; https://drms.colorado.gov/rules-and-regulations

strength (in this case, primarily the weak interbed), water pressure within the joint, joint orientation and slope geometry intersections within a Monte Carlo sampling method.

For the East Face, a failure plane angle of [redacted], which correlates to the bedding dip of the interbed material, was analyzed. The friction angle of [redacted] and cohesion of [redacted] was realistically used in our analysis. The KUE stability analyses had reduced the cohesion value of [redacted], which was determined through empirical means and calibrated through a backanalysis of the west face ground event, by approximately 50% to [redacted] as an additional level of conservatism for the "critical structures". For the remediation stabilization that additional level of conservatism is no longer relevant, therefore the determined cohesion value of [redacted] was used in this analysis. A tension crack was placed at the calculated minimum facto of safety at an angle of 45 degrees, as mapped on the east face. Critically, water pressure was deterministically modeled as 30% filled with peak pressure at the tension crack base, in line with previous stability analyses. Using the back-calculated cohesion from the previous models, based on the failure, the long-term factor of safety for the East Face is **1.75** and the psuedostatic seismic factor of safety is **1.38**. Therefore, no active stabilization is required for the remediation of the East Face, as demarked on Figure 1.

For the West Face, encompassing a length of 450 linear feet, with an upper limestone height of [redacted], solid thread bar anchors were analyzed. [redacted] achieves a long-term factor of safety of **1.51** and the psuedostatic seismic factor of safety of **1.15**. Following DRMS MLRB policies, the minimum seismic FOS controls the anchor design within the West Face.

[redacted]	[redacted]	[redacted]	[redacted]	[redacted]	[redacted]
[redacted]	[redacted]	[redacted]	[redacted]	[redacted]	[redacted]

Blasting impacts were not analyzed due to the analyses being performed for post-mining periods.

Closure

I appreciate the opportunity to present these findings. Should you have any further questions or need additional information please feel free to contact me.

GOLDEN GEOTECHNICS, INC.

Sean Sundermann, PG, CEG
Principal Geologist / President

August 8, 2025

Mr. Robert Wagner
RMR Aggregates, Inc.
6200 S. Syracuse Way, Ste. 450
Greenwood Village, CO 80111

RE: **Highwall Reclamation Seismic Factor of Safety**
Mid-Continent Limestone Quarry
Glenwood Springs, CO
GGI Project No.: 23003-04

Dear Mr. Wagner,

Golden Geotechnics, Inc. (GGI), in conjunction with Brierley Associates, Corp (BAC) responds herein to a request for information by the State of Colorado Division of Reclamation, Mining and Safety (DRMS) in Technical Revision (TR-9) Adequacy Review 2 as part of the Mid-Continent Limestone Quarry interim highwall reclamation stabilization plan.

The technical reviewer requested to "provide details and rationale regarding the seismic parameters applied to the pseudo-static models". There is currently no applicable code or standard design guidelines regarding seismic design of rock slopes that include factors of safety for the seismic limit state. In lieu of this, we believe that the published guideline most relevant to rock slopes is the Geotechnical Engineering Circular No. 7 on soil nail wall design from the Federal Highway Administration (FHWA). The current version, released in 2015, includes the following recommendations highlighted in yellow:

Table 5.1: Minimum Recommended Factors of Safety for the Design of Soil Nail Walls Using the ASD Method (1)

Limit State	Condition	Symbol	Minimum Recomm. Factors of Safety, Static Loads	Minimum Recomm. Factors of Safety, Seismic Loads
Overall	Overall Stability	FS_{OS}	1.5(2)	1.1(6)

(6) The minimum FS_{OS} for seismic overall stability should be 1.0, when horizontal seismic coefficients are used and these were derived from estimated, allowable seismic deformations.

Soil nail walls still need to be fully designed for seismic loads when applicable. AASHTO (2014), Article 11.5.4.2, gives guidance about when earth-retaining structures require analysis for loads at the Extreme-Event I limit state. The criteria for MSE walls, another earth-supporting system regarded as flexible, can be considered appropriate for soil nail walls. It is stated in that article that a seismic design is not mandatory for MSE walls in AASHTO-defined Seismic Zone 1 (i.e., for a design response acceleration for 1-sec period, $S_{1D} \leq 0.15$), Seismic Zone 2 ($0.15 < S_{1D} \leq 0.30$), and Seismic Zone 3 ($0.30 < S_{1D} \leq 0.50$), or for walls at sites where the site-adjusted, design PGA_D is ≤ 0.4 g, and as long as the soils behind the wall do not liquefy (mainly cohesionless soils), or sustain significant strength degradation (mainly fine-grained soils) during dynamic loading.

6.8.4 Step 9c Evaluate overall stability with seismic loads

Perform overall stability analysis as discussed in previous sections while adding the seismic coefficient, k_h, as input. Use minimum factors of safety for seismic conditions as included in Table 5.1. Verify that the computed factors of safety for seismic conditions are met. If the minimum criterion is not met, modify nail length, and/or other design parameters as applicable and repeat analysis until the calculated factors of safety exceed minimum recommended values.

Because significant seismic events are relatively rare, they are considered extreme load cases in civil engineering and are therefore compared to relatively low factors of safety relative to more frequent loads such as standard live and dead loads. The design factors of safety shown above in Table 5.1 for the "Overall" or global stability condition are for static and seismic (pseudo-static) load cases. The minimum factor of safety for the static case is 1.5, which is also commonly used in the industry for stability of permanent slopes, even without retaining walls or active stabilization. When a seismic horizontal load is added to the slope stability pseudo-static calculation, the resulting factor of safety is reduced from the additional load but because the design seismic event is so infrequent, the minimum required factor of safety is also reduced from 1.5 to 1.1 (or sometimes 1.0 if the seismic load is evaluated based on site-specific seismic deformation conditions). Colorado DOT's minimum acceptable factors of safety reflect these values for slopes and retaining walls as 1.3 static long term and 1.1pseudo-static seismic for slopes, but CDOT takes another step in acknowledging Colorado's low seismic potential and removes the minimum pseudo-static FOS for natural slopes "where failure or significant deformation will affect adjacent bridges or critical facilities."

The FHWA, American Association of State Highway Officials (AASHTO) and the American Society of Civil Engineer (ASCE) general seismic design guidelines identify the area where the Mid-Continent Quarry is located as a low seismic hazard zone. These guidelines separate the country into different seismic zones through using the design spectral acceleration coefficient at 1.0 second, or S_{1D}. This parameter is used in civil and structural design to represent the anticipated acceleration a structure will experience during an earthquake at a specific period. As part of site stability calculations, a longer period acceleration of 1.0 seconds was used, as appropriate for a natural slope. The design site-corrected response acceleration calculated for the site is 0.07 (Appendix G of the March 2024 report). That value places the site in the lowest hazard zone, Zone 1. In fact the value is less than half of the boundary value of Zone 2. The statement below from Geotechnical Engineering Circular No. 7 provides the zones and some context to the site S_{1D} value of 0.07.

> walls. It is stated in that article that a seismic design is not mandatory for MSE walls in AASHTO-defined Seismic Zone 1 (i.e., for a design response acceleration for 1-sec period, $S_{1D} \leq 0.15$), Seismic Zone 2 ($0.15 < S_{1D} \leq 0.30$), and Seismic Zone 3 ($0.30 < S_{1D} \leq 0.50$), or for walls at sites where the site-adjusted, design PGA_D is ≤ 0.4 g, and as long as the soils behind the wall do not liquefy (mainly cohesionless soils), or sustain significant strength degradation (mainly fine-grained soils) during dynamic loading.

Further, this is another design guideline that does not recommend pseudo-static design in Seismic Zone 1.

The earthquake record in Colorado is so dearth of significant events that critical facilities around the state which require a site-specific seismic hazard assessment are left studying historic records for subjective “felt” reports of earthquakes. The only significant event was November of 1882. The earthquake was not recorded but records indicate some intensity was felt in northern Colorado and Wyoming. The seismic source model for the seismic hazard assessment completed for the Rocky Flats facility placed the epicenter in northwestern Colorado with a magnitude of ML 6.5 and an Intensity scale at the Mid-Continent site of V (trees and bushes shaken noticeably). Later studies by the USGS indicate that the 1882 earthquake did not rupture the ground surface.

The FHWA recommended set of safety factors are not dependent on a determination of slope criticality. However, the similarity of the seismic minimum factor of safety of 1.1 to the Colorado Department of Reclamation Mining and Safety (DRMS) 2018 Policies of the Mined Land Reclamation Board requirement for pseudo-static analysis of non-critical slopes of 1.15 suggests a corresponding concurrence of required slope stability for the post-mining case where potential slope failure would not result in damage to any structures or improvements due to the remoteness of the site. In our opinion, this agreement is consistent with our understanding of the standard practice in this area of geotechnical engineering.

Given the analysis and factors outlined in this letter, it is our recommendation that the use of the non-critical DRMS pseudo-static factor of safety for the post-mining configuration at this site is appropriate. Accordingly, we propose the DRMS view the 1.15 pseudo-static factor of safety as acceptable.

Closure

We appreciate the opportunity to respond to the RFI from DRMS. Should you have any further questions or need additional information please feel free to contact me.

GOLDEN GEOTECHNICS, INC.

Sean Sundermann, PG, CEG
Principal Geologist / President

BRIERLEY ASSOCIATES, INC.

Bill Zietlow, P.E.
Principal Engineer

RocPlane Analysis Information
Mid-Continent Mine - West Face

Project Summary

File Name	East wall - at failure 15 ft
Project Title	Mid-Continent Mine - West Face
Analysis	Planar Analysis
Author	Kyle Friedman
Company	Brierley Associates
Date Created	6/22/2023, 8:54:59 AM

Analysis Results

Analysis Type - Deterministic

Normal Force	32394.8 lbs/ft
Driving Force	19469.8 lbs/ft
Resisting Force	34086.6 lbs/ft
Factor of Safety	1.75074

Geometry

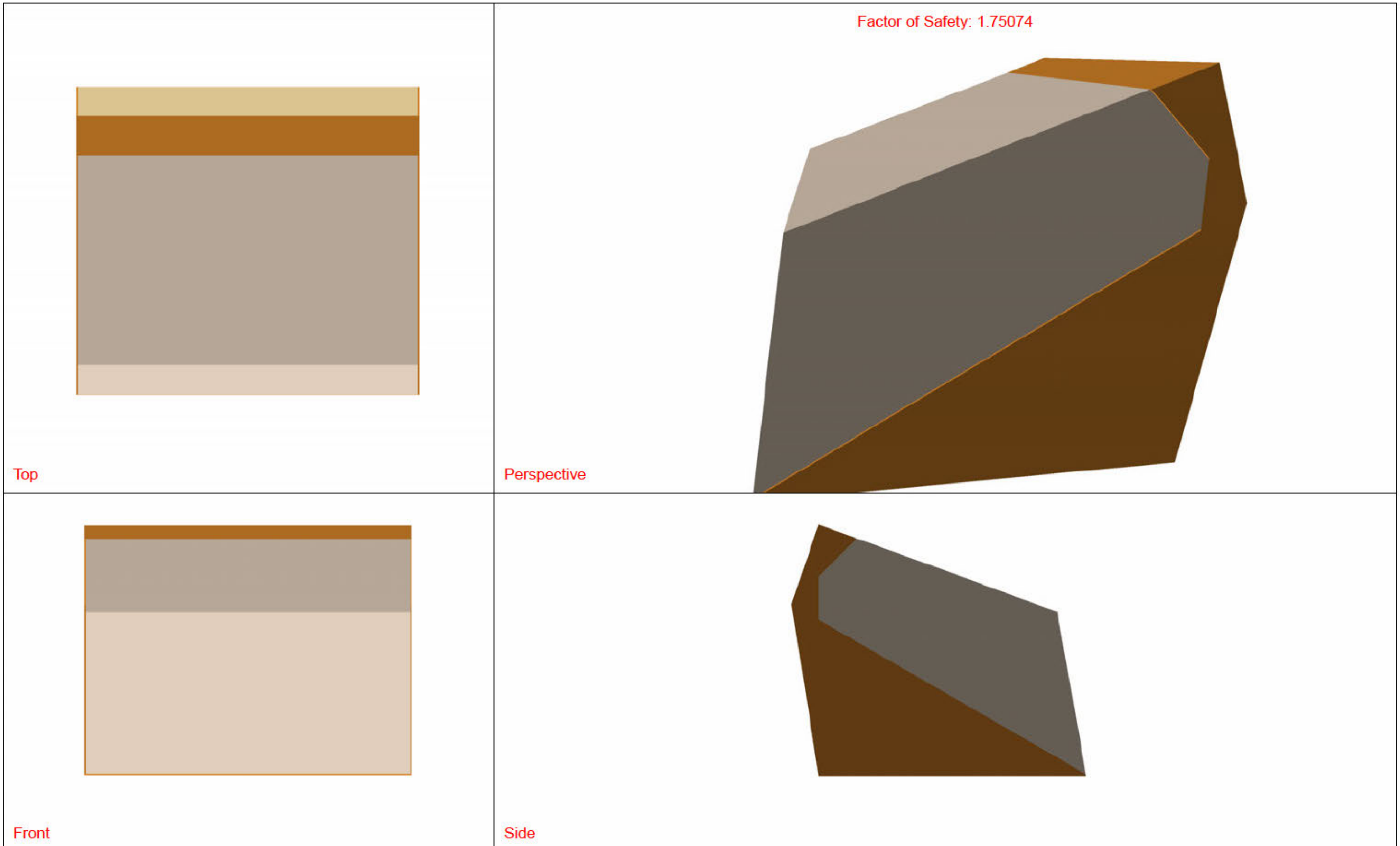
[redacted]	[redacted]
Failure Plane Angle	30 °
Upper Face Angle	20 °
Bench Width	Not Present
Waviness	3 °
Intersection Point (B) of Slope and Upper Face	(2.6449 , 15)
Intersection point (C) of Tension Crack and Upper Face	(21.1793 , 21.746)
Intersection point (D) of Failure Plane and Tension Crack	(27.2135 , 15.7117)
Slope Length (Origin --> B)	15.2314 ft
Tension Crack Length (C --> D)	8.53369 ft
Failure Plane Length (Origin --> D)	31.4234 ft
Tension Crack	Present
Tension Crack Angle	135 °
Distance From Crest	18.5344 ft
Tension Crack Length	6.03423 ft

Strength

Shear Strength Model	Mohr-Coulomb
[redacted]	[redacted]

Plane Water Pressure

Water Unit Weight	62.4 lbs/ft3
Pressure Distribution Model	Peak Pressure - Mid Height
Percent Filled	30
Water Force on Failure Plane	1327.87 lbs/ft
Water Force on Tension Crack Plane	0 lbs/ft



rocscience

ROCPLANE 4.013

Project	Mid-Continent Mine - West Face		
Analysis Description	Planar Analysis		
Drawn By	Kyle Friedman	Company	Brierley Associates
Date	6/22/2023, 8:54:59 AM	File Name	East wall - at failure 15 ft.pln4

RocPlane Analysis Information
Mid-Continent Mine - West Face

Project Summary

File Name	East wall - at failure 15 ft - Sesimic
Project Title	Mid-Continent Mine - West Face
Analysis	Planar Analysis
Author	Kyle Friedman
Company	Brierley Associates
Date Created	6/22/2023, 8:54:59 AM

Analysis Results

Analysis Type - Deterministic

Normal Force	29941.6 lbs/ft
Driving Force	23718.8 lbs/ft
Resisting Force	32814.1 lbs/ft
Factor of Safety	1.38346

Geometry

[redacted]	[redacted]
Bench Width	Not Present
Waviness	3 °
Intersection Point (B) of Slope and Upper Face	(2.6449 , 15)
Intersection point (C) of Tension Crack and Upper Face	(21.1793 , 21.746)
Intersection point (D) of Failure Plane and Tension Crack	(27.2135 , 15.7117)
Slope Length (Origin --> B)	15.2314 ft
Tension Crack Length (C --> D)	8.53369 ft
Failure Plane Length (Origin --> D)	31.4234 ft
Tension Crack	Present
Tension Crack Angle	135 °
Distance From Crest	18.5344 ft
Tension Crack Length	6.03423 ft

Strength

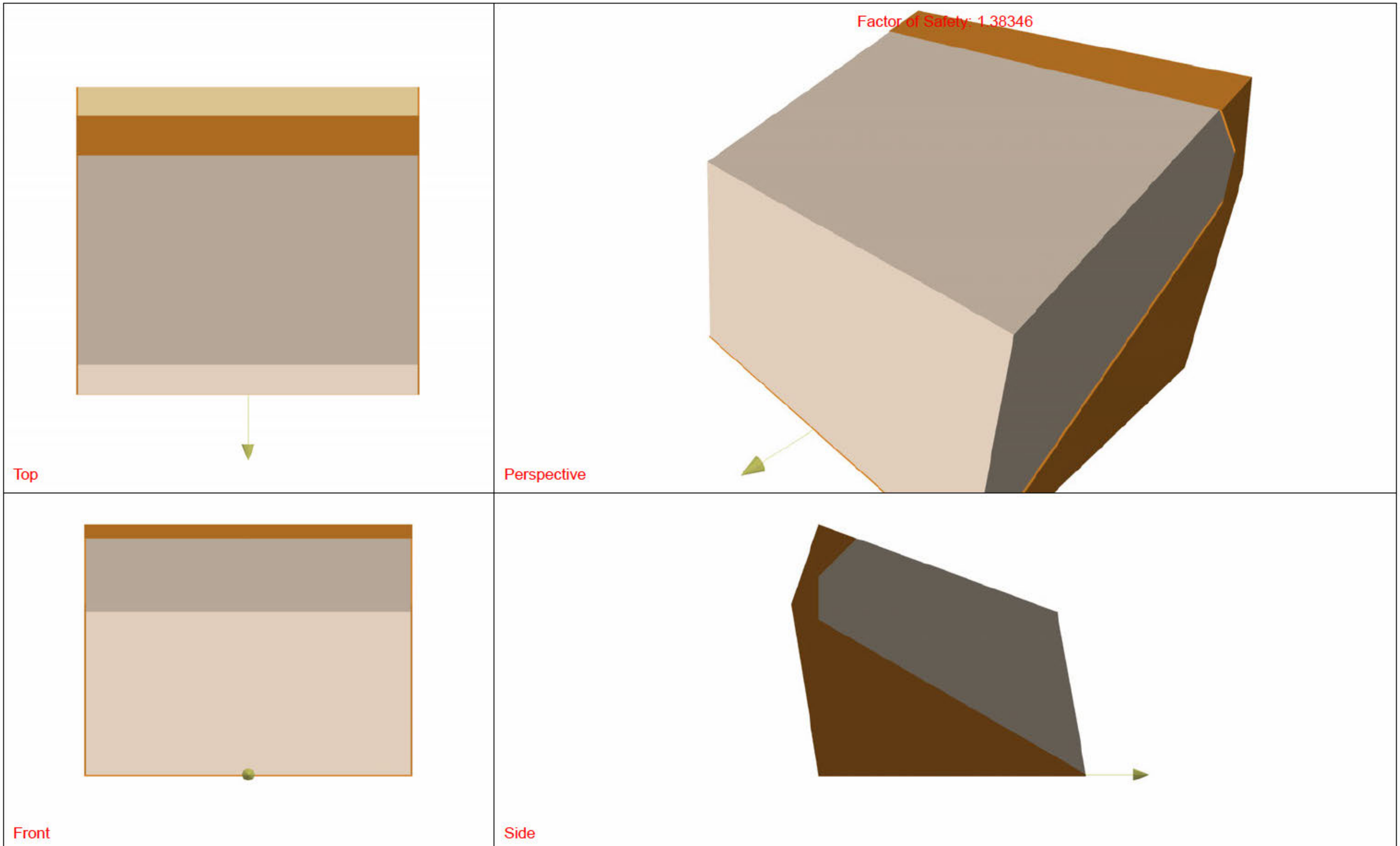
Shear Strength Model	Mohr-Coulomb
[redacted]	[redacted]

Plane Water Pressure

Water Unit Weight	62.4 lbs/ft3
Pressure Distribution Model	Peak Pressure - Mid Height
Percent Filled	30
Water Force on Failure Plane	1327.87 lbs/ft
Water Force on Tension Crack Plane	0 lbs/ft

Seismic Force

Direction	Horizontal
Seismic Coefficient	0.126
Seismic Force	4906.38 lbs/ft



rocscience

ROCPLANE 4.013

Project	Mid-Continent Mine - West Face		
Analysis Description	Planar Analysis		
Drawn By	Kyle Friedman	*Company*	Brierley Associates
Date	6/22/2023, 8:54:59 AM	*File Name*	East wall - at failure 15 ft - Sesimic.pln4

[redacted]

RMR Aggregates, Inc. Technical Revision 9 Redaction Privilege Log

Applies to TR-9 Submission dated October 3, 2025

Redaction #	Applicable Statute	Reason for Confidentiality
1	HRA 34-32-112, Div. Rule 1.3(3)	Describes information related to the ore body
2	CRS 24-72-204(3)(a)(IV)	These are specifically engineered parameters based off geological data. This is prvileged and geological data.
3	HRA 34-32-112, Div. Rule 1.3(3)	Describes information related to the ore body
4	CRS 24-72-204(3)(a)(IV)	These are specifically engineered parameters based off geological data. This is prvileged and geological data.
5	HRA 34-32-112, Div. Rule 1.3(3)	Shows information related to the ore body. Specifically, the point where the upper limestone layer ends.
6	HRA 34-32-112, Div. Rule 1.3(3)	Describes information related to the ore body
7	HRA 34-32-112, Div. Rule 1.3(3)	Describes information related to the ore body
8	HRA 34-32-112, Div. Rule 1.3(3)	Describes information related to the ore body
9	HRA 34-32-112, Div. Rule 1.3(3)	Describes information related to the ore body
10	HRA 34-32-112, Div. Rule 1.3(3)	Describes information related to the ore body
11	HRA 34-32-112, Div. Rule 1.3(3)	Describes information related to the ore body
12	HRA 34-32-112, Div. Rule 1.3(3)	Describes information related to the ore body
13	CRS 24-72-204(3)(a)(IV)	These are specifically engineered parameters based off geological data. This is prvileged and geological data.
14	HRA 34-32-112, Div. Rule 1.3(3)	Describes information related to the ore body
15	CRS 24-72-204(3)(a)(IV)	These are specifically engineered parameters based off geological data. This is prvileged and geological data.
16	HRA 34-32-112, Div. Rule 1.3(3)	Describes information related to the ore body
17	HRA 34-32-112, Div. Rule 1.3(3)	Describes information related to the ore body
18	HRA 34-32-112, Div. Rule 1.3(3)	Describes information related to the ore body
19	HRA 34-32-112, Div. Rule 1.3(3)	Describes information related to the ore body
20	CRS 24-72-204(3)(a)(IV)	The information in this quote is priviledged and confidential due to the financial quote information contained within. Additionally, RMRA does not have permission from the quoting company to release the information they provided.
21	CRS 24-72-204(3)(a)(IV)	The information in this quote is priviledged and confidential due to the financial quote information contained within. Additionally, RMRA does not have permission from the quoting company to release the information they provided.